OVERVIEW OF THE MEPDG (2002) PAVEMENT DESIGN GUIDE

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Acknowledgements

Guide for Design of New and Rehabilitated Pavements Structures (NCHRP 1-37A and 1-40D).

- Applied Research Associates, Inc. (John Hallin, Michael Darter, & many others)
- Arizona State University (Prof. Matt Witczak, Mohamed El-Basyouny, & many others)
- University of Maryland
- Several consultants around the world

Many slides in this presentation were developed under the above projects

Outline

- Background of the Design Guide
- Overview of the MEPDG
- Examples of Minnesota Experience
- Current Status and Summary

Background of the Design Guide

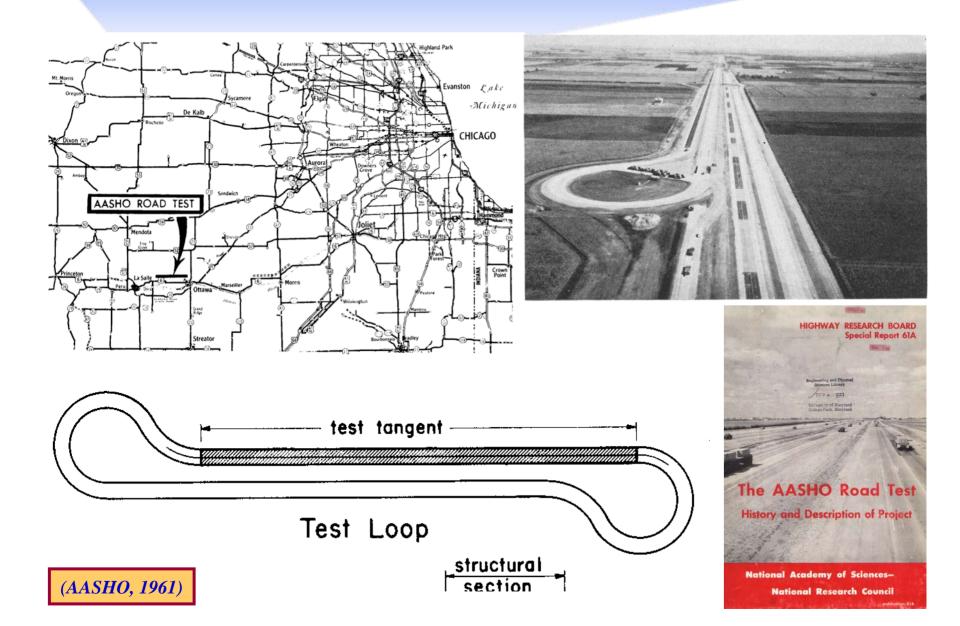
- History of the current design procedure
- Limitation of the current design guide
- Needs for new design procedure
- Background of the new mechanisticempirical design procedure

History of the Current AASHTO Pavement Design Guide

- Empirical design methodology based on AASHO Road Test in the late 1950's
- Several versions:
 - 1961 (Interim Guide), 1972, 1986
 - 1986 version included refined materia characterization
 - 1993 revised version
 - More on rehabilitation
 - More consistency between flexible, rigid designs
 - Current version for flexible design procedures



AASHO Road Test (late 1950's)

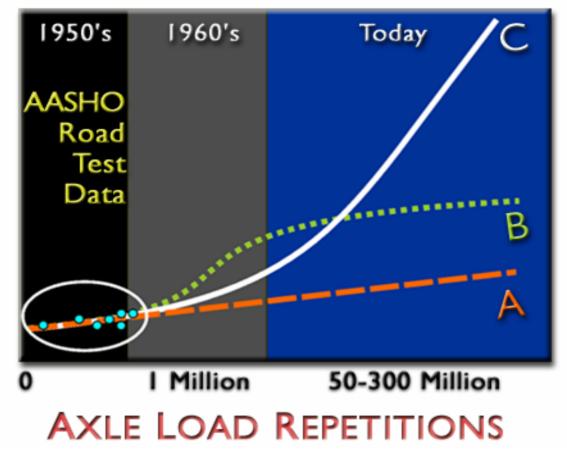


AASHO Road Test Location

Ottawa, IL

Low Traffic Levels...

PAVEMENT THICKNESS

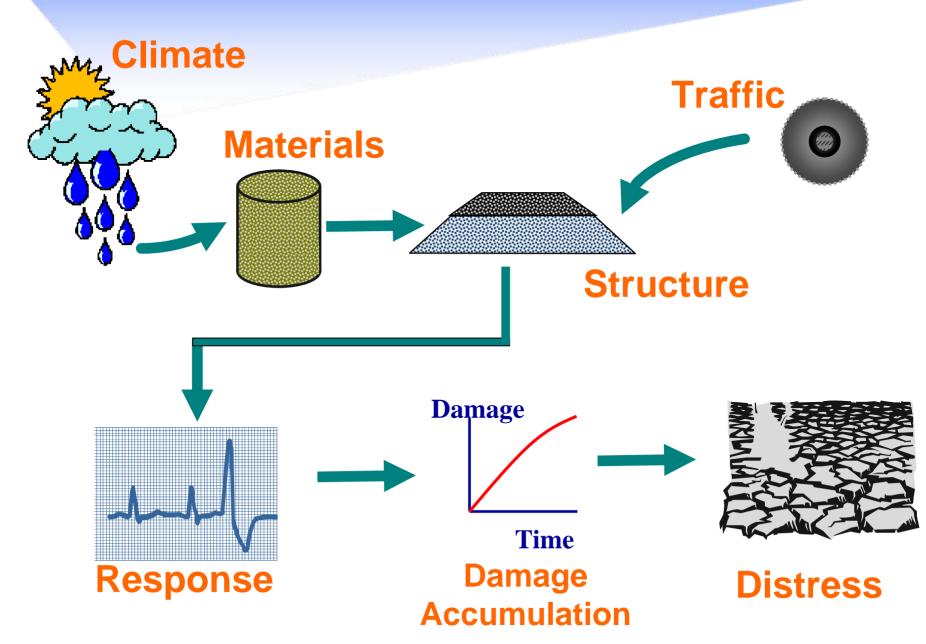


NCHRP Project 1-37A Objective

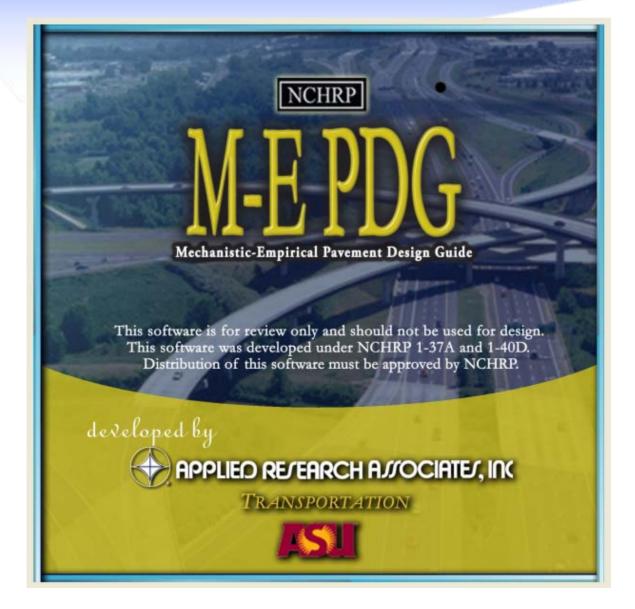
Develop and deliver:

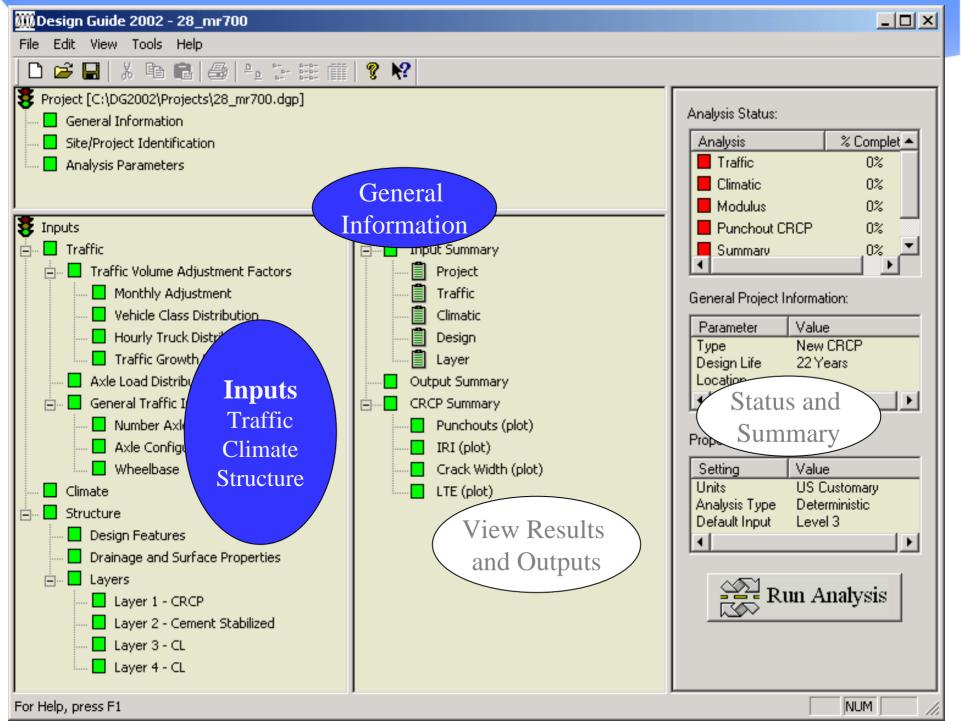
Guide for Design of New and Rehabilitated Pavements Structures, based on existing <u>mechanistic-empirical</u> <u>technology</u>, accompanied by the necessary computational software, for adoption and distribution by AASHTO.

Overview of the MEPDG



Design Guide Software





Key Advantage of M-E Design

"Comprehensive" design procedure: Not Just Thickness!

M-E models directly consider true effects and interactions of inputs on structural distress and ride quality.

Design optimization possible where all distress types are minimized!

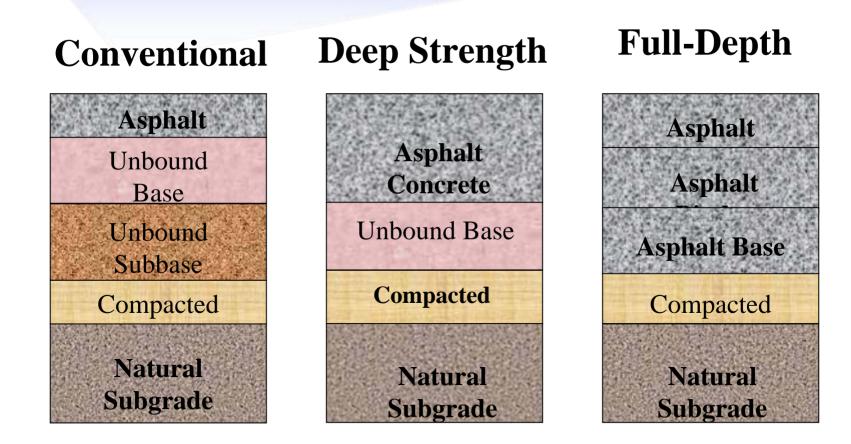






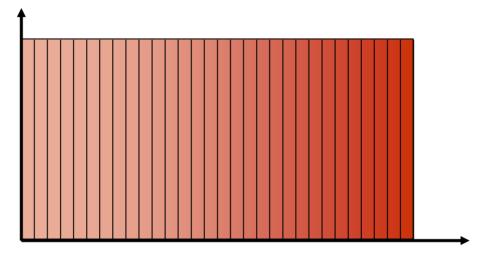


Flexible Pavements



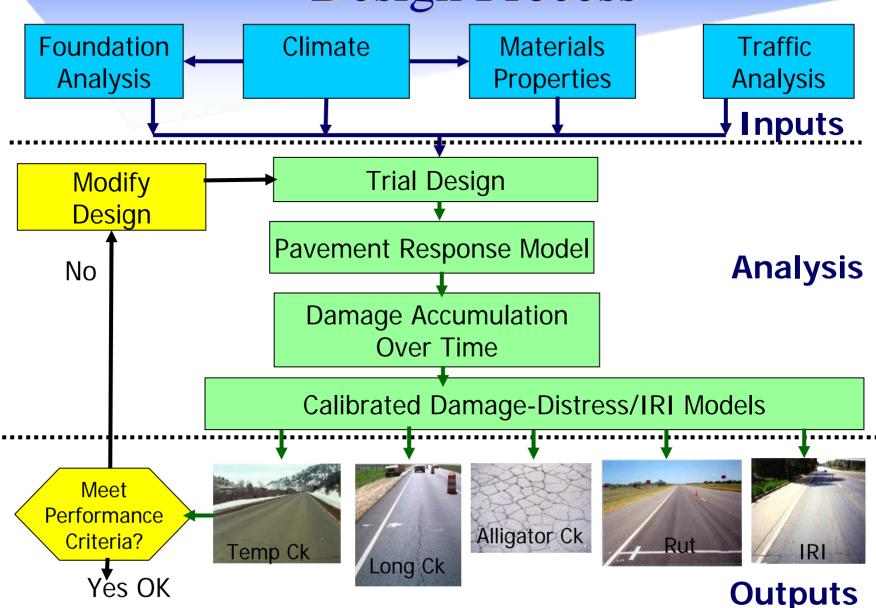
Damage Accumulation - Incremental Damage Concept

- Design life is divided into time increments of:
 - 1 month for rigid pavements
 - 15 days for flexible pavements



Design life

Design Process



Design Inputs – 3 Main Categories

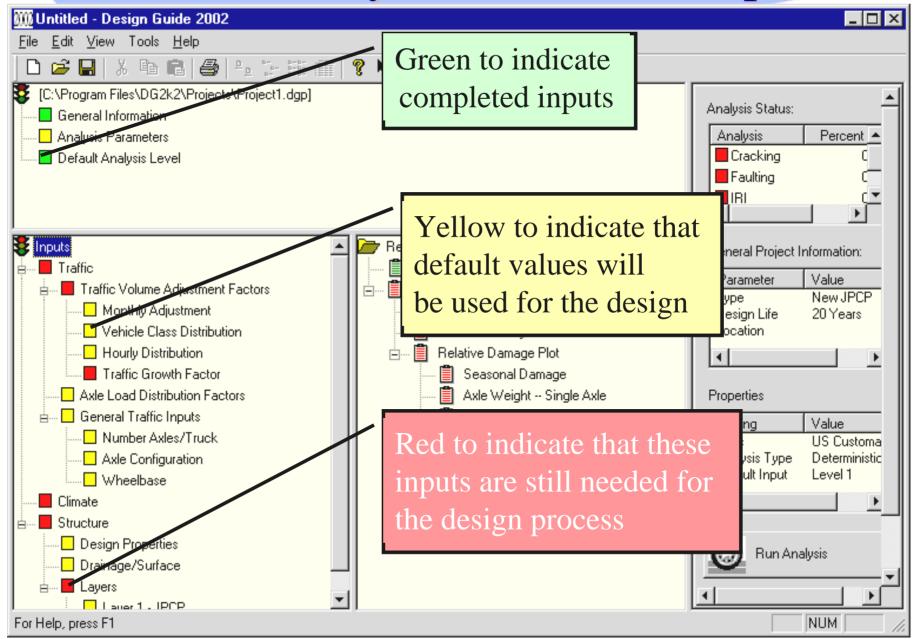
- Traffic
 - Volume
 - Axle load distribution
 - Axle configuration
- Climate
 - Latitude, longitude, elevation, etc.
- Structure
 - Layers, thicknesses, and material properties

Input Levels

The Design Guide includes three levels to obtain inputs to facilitate use and implementation.

Input Level	Determination of Input Values	Knowledge of Input Parameter
1	Project/Segment Specific Measurements—Lab, WIM, FWD	Good
2	Correlations/Regression Equations, Regional Values— CBR, R-Value, Dynamic Cone Penetrometer	Fair
3	Soil Classifications, Typical values	Fair - Poor

User-Friendly Color-Coded Inputs

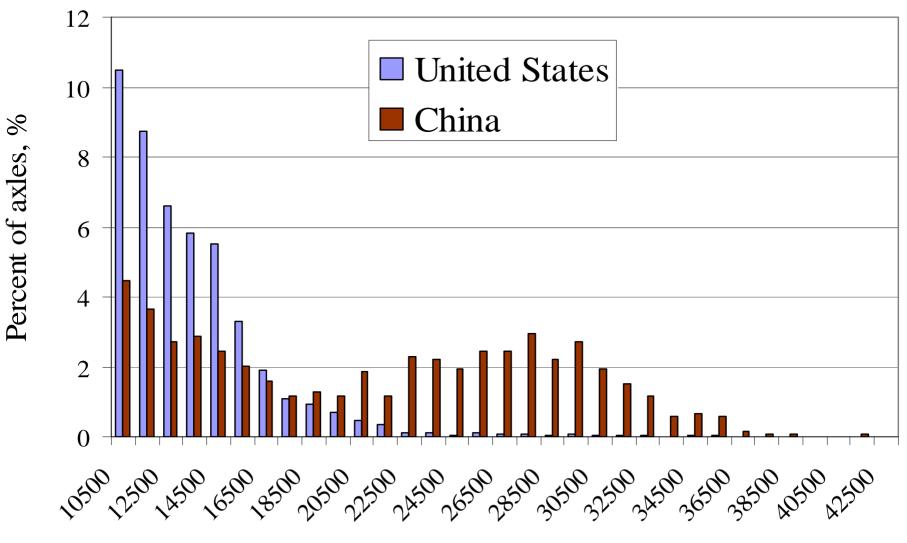


Traffic Loading Inputs

- Vehicle volume, growth & classification
- Single, tandem, tridem, quad axle load distributions
- Monthly vehicle distribution
- Hourly load distribution
- Lateral lane distribution
- Tire pressure
- Tractor wheelbase
- Truck speed



Axle Load Spectrum (Single Axles)



Axle load

Climate Inputs

- Hourly climatic data: Weather Stations
 - Temperature
 - Precipitation
 - Wind speed
 - Cloud cover
 - Relative ambient humidity
- Water table level



Simplified Climatic Inputs

Environment/Climatic

?×

- Climatic data for a specific weather station.
- Interpolate climatic data for given location.

-93.14Longitude (degrees.minutes) Elevation (ft) Seasonal Depth of water table (ft) Annual average Note: Ground water table depth is a positive number measured from the pavement surface. Select weather station BAUDETTE, MN ~ BRAINERD, MN DULUTH, MN HIBBING, MN INTERNATIONAL FALLS, MN MINNEAPOLIS, MN MINNEAPOLIS, MN MINNEAPOLIS, MN PARK RAPIDS, MN REDWOOD FALLS, MN ROCHESTER, MN ST CLOUD, MN ¥

Latitude (degrees.minutes)

Station Location: MINPLIS-ST PAUL INTL ARPT Months of available data:116 Months missing in file:0

44.53

Select Station

Cancel

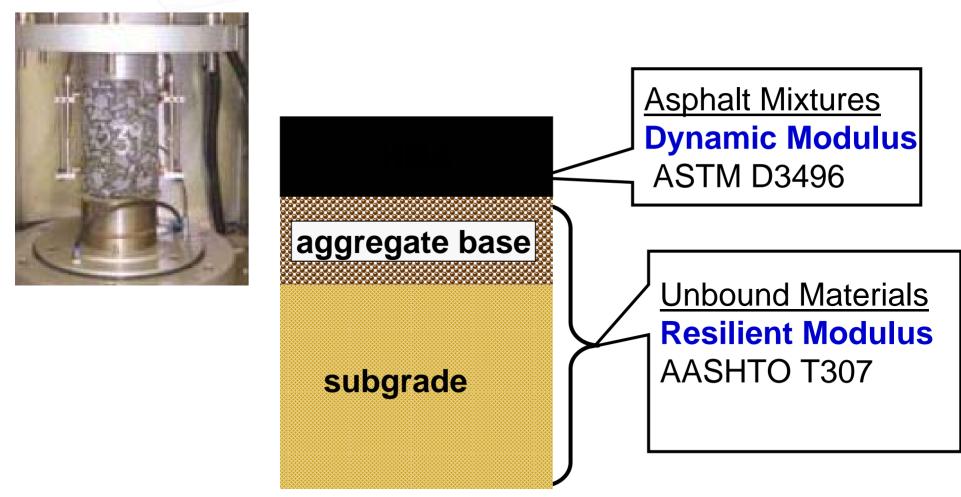
Materials Inputs

Dynamic Modulus of HMA

Resilient Modulus of Unbound Materials

Material modulus & volumetrics are key properties.

Layer Materials Characterization



Empirical Relation for |E*|

$$\log E = -1.249937 + 0.29232 \rho_{200} - 0.001767 (\rho_{200})^2 - 0.002841 \rho_4 - 0.058097 V_a$$
$$-0.802208 \left(\frac{V_{beff}}{V_{beff} + V_a} \right) + \frac{3.871977 - 0.0021 \rho_4 + 0.003958 \rho_{38} - 0.000017 (\rho_{38})^2 + 0.005470 \rho_{34}}{1 + e^{(-0.6033^{\circ}3 - 0.313351 \log(f) - 0.393532 \log(\eta))}}$$
(2.3)

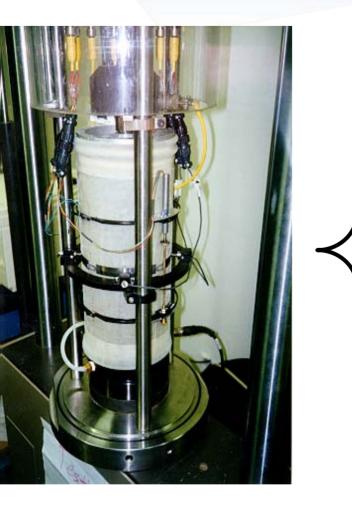
where:

Ε	#	Dynamic modulus, 10 ⁵ psi
η	=	Bitumen viscosity, 10 ⁶ Poise
f	=	Loading frequency, Hz
Va	=	Air void content, %
V_{beff}	=	Effective bitumen content, % by volume
ρ ₃₄	=	Cumulative % retained on the 19-mm sieve
ρ ₃₈	=	Cumulative % retained on the 9.5-mm sieve
ρ4	=	Cumulative % retained on the 4.76-mm sieve
ρ ₂₀₀		% passing the 0.075-mm sieve (Witczak <i>et a</i>

Simplified Asphalt Materials Inputs (Level 3)

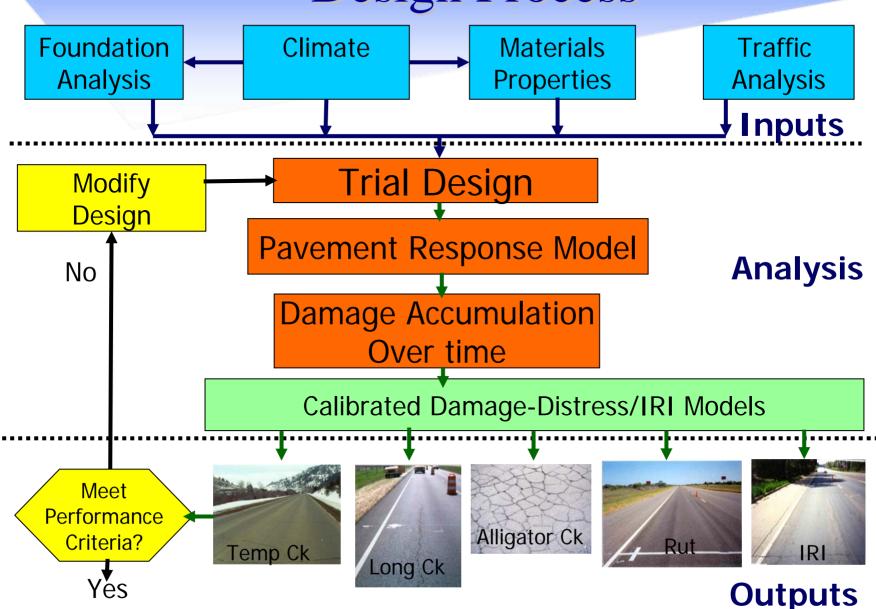
Asphalt Material P	roperties						?	3				
Level: 3		t material typ hickness (in):	. , .	halt concr 10	rete		•	Asphalt Material Pro	operties Asphalt material type:	Asphalt concrete		2×
Asphalt Mix	Options (•	der Asp Superpave Convention Convention	binder grad al viscosity	ding grade				General	Layer thickness (in): Asphalt Binder Asphalt	Poisson's Ratio —	e model to	
High Temp (°C) -1 46 -1 52 -5 58 -6 64 -1 70 -1 76 -1 82 -1	A 9.46	-22		(°C) -34	-40	-46		Them	ies as Built ontent (%): 11 8.5		e model to son's ratio.	
									🗸 ок	🗶 Cancel		

Materials Testing: Unbound Base/Soils

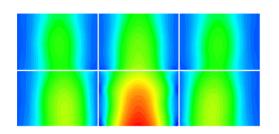


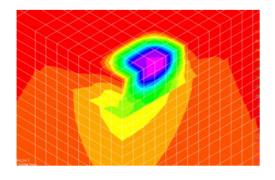
- Resilient modulus, Mr
 - -Laboratory
 - Back-calculated from deflection data
 - Or estimate from:
 - -CBR,
 - -R-Value, or
 - Dynamic Cone Penetration
 - -Soil Classification

Design Process



Structural Response Models





- For rigid pavements
 - ISLAB2000—Finite Element Model (FEM) program
- For flexible pavements
 > JULEA—Linear elastic layered analysis program

Cumulative Incremental Damage Approach

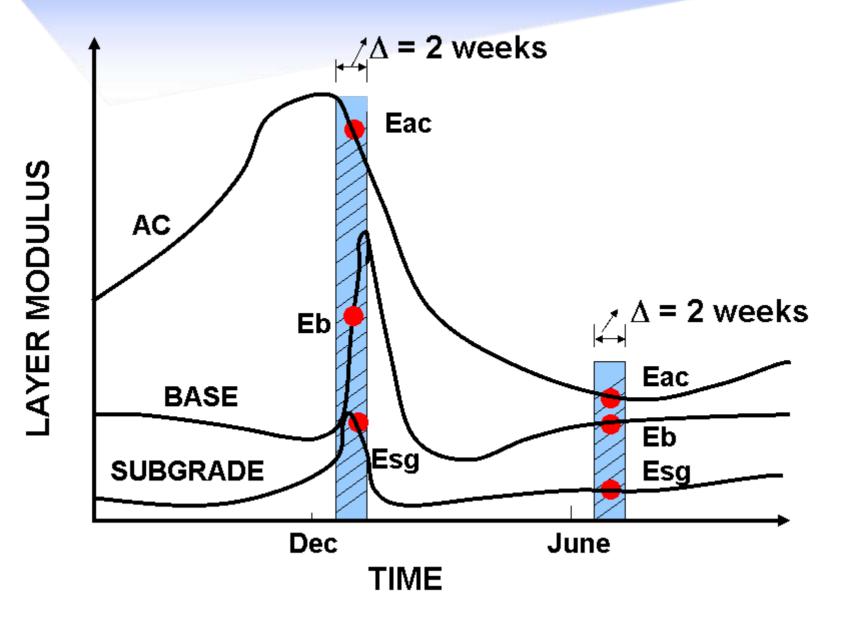
- Changes over time
 - -Material strength and stiffness
 - Aging of asphalt
 - Moisture changes of soils & aggregates
 - Temperature changes & freezing
 - -Traffic hourly, seasonally, and growth over time

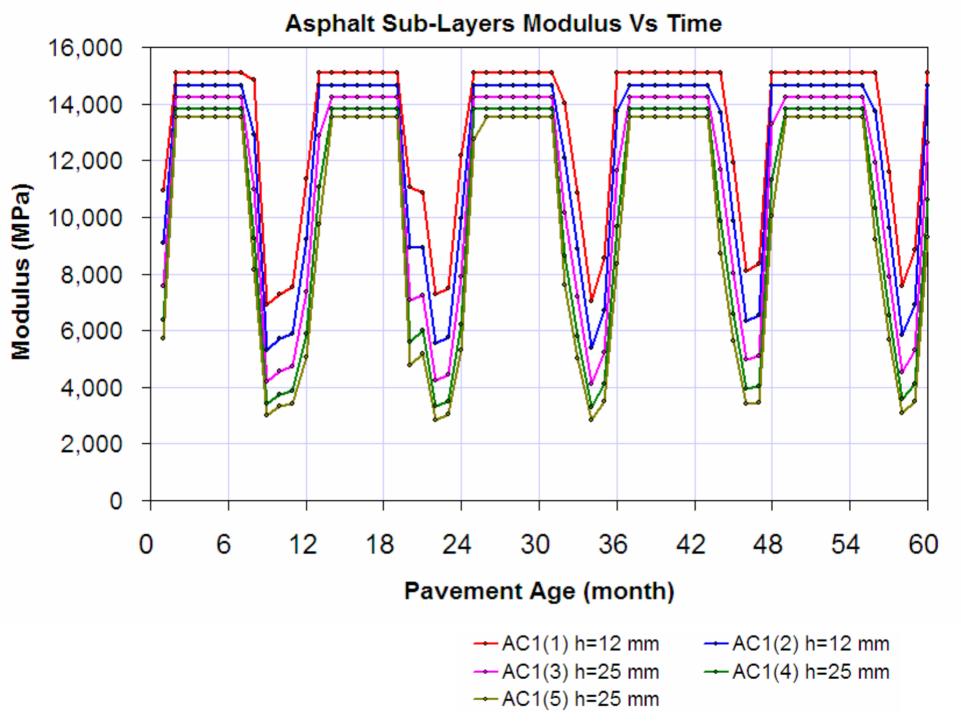
Matrix of Adjustment Coefficients For Resilient Moduli of Unbound Layers



	Time	(days)												
Nodes	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
1															AC
2															
3	50	50	50	50	50	50	50	50	0.7	0.7	0.7	0.7	0.7	0.7	BASE
4	50	50	50	50	50	50	50	50	0.7	0.7	0.7	0.7	0.7	0.7	
5	50	50	50	50	50	50	50	0.7	0.7	0.7	0.7	0.7	0.7	0.7	
6	50	50	50	50	50	50	50	0.7	0.7	0.7	0.7	0.7	0.7	0.7	
7	50	50	50	50	50	50	50	0.7	0.7	0.7	0.7	0.7	0.7	0.7	
8	50	50	50	50	50	50	50	0.7	0.7	0.7	0.7	0.7	0.7	0.7	
9	75	75	75	75	75	75	75	0.6	0.6	0.6	0.6	0.6	0.6	0.6	SUBBASE
10	75	75	75	75	75	75	75	0.6	0.6	0.6	0.6	0.6	0.6	0.6	
11	75	75	75	75	75	75	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.7	
12	75	75	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.7	0.7	0.7	0.7	0.7	
13	75	0.6	<i>0.6</i>	0.6	0.6	0.6	0.6	0.6	0.7	0.7	0.7	0.7	0.7	0.7	
14	0.8	0.8	0.8	0.8	0.9	0.9	0.9	0.9	0.9	0.9	0.9	1	1	1	
15	0.8	0.8	0.8	0.9	0.9	0.9	0.9	0.9	0.9	0.9	1	1	1	1	
16	0.8	0.9	0.9	0.9	0.9	0.9	0.9	0.9	1	1	1	1	1	1	
17	0.8	0.8	0.8	0.8	0.8	0.9	0.9	0.9	0.9	0.9	0.9	0.9	0.9	0.9	SUBGRADE
18	0.7	0.7	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	
19	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	
20	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	
21	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	
22	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	
23	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	
24	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	

Annual Modulus Variability





Flexible Pavement Performance

Fatigue Cracking

hermal

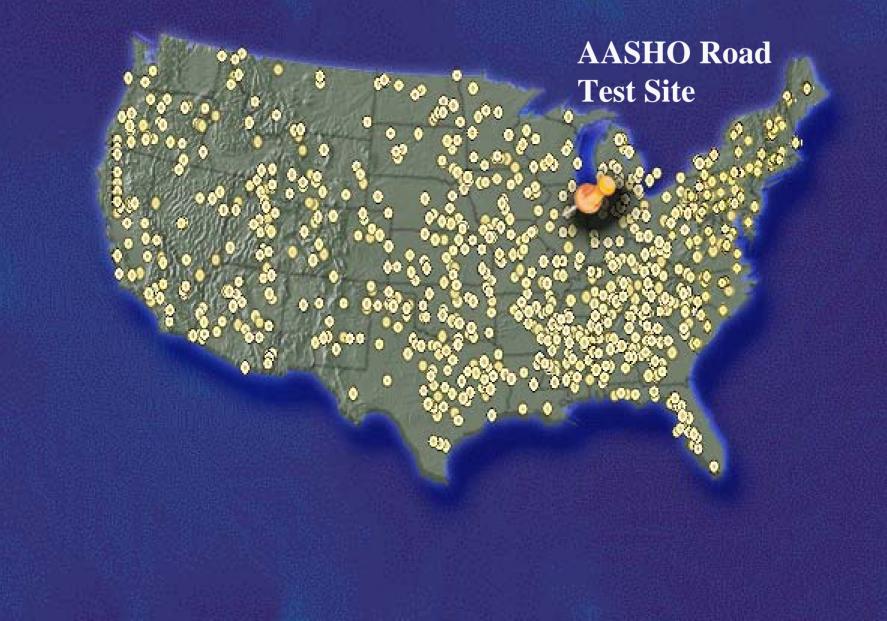
Cracking

Longitudinal Cracking



IRI

Field Performance - The LTPP Study



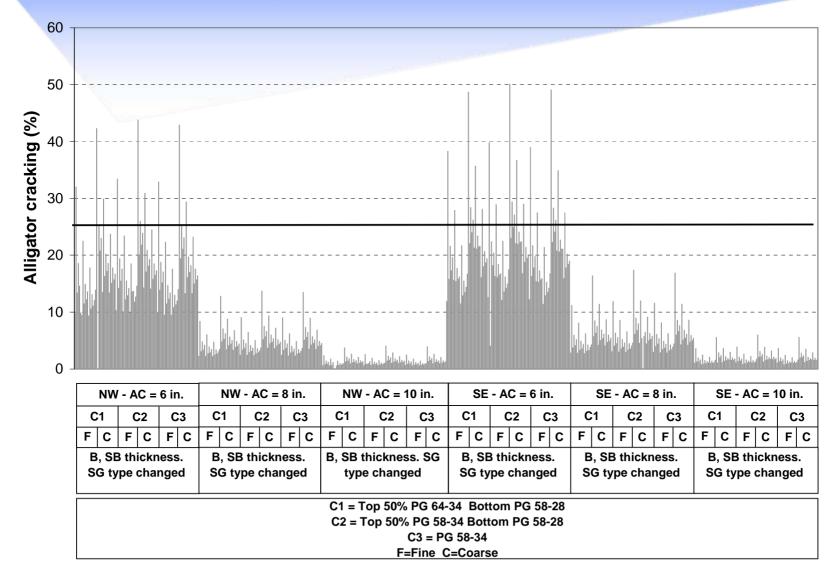
Implementation of Design Guide

- Procedures to obtain each input
 - Traffic, climate, materials, structure, rehabilitation
- Conduct sensitivity studies
- Develop case studies of existing pavements
- Local calibration (varying climates within country) of distress & IRI models

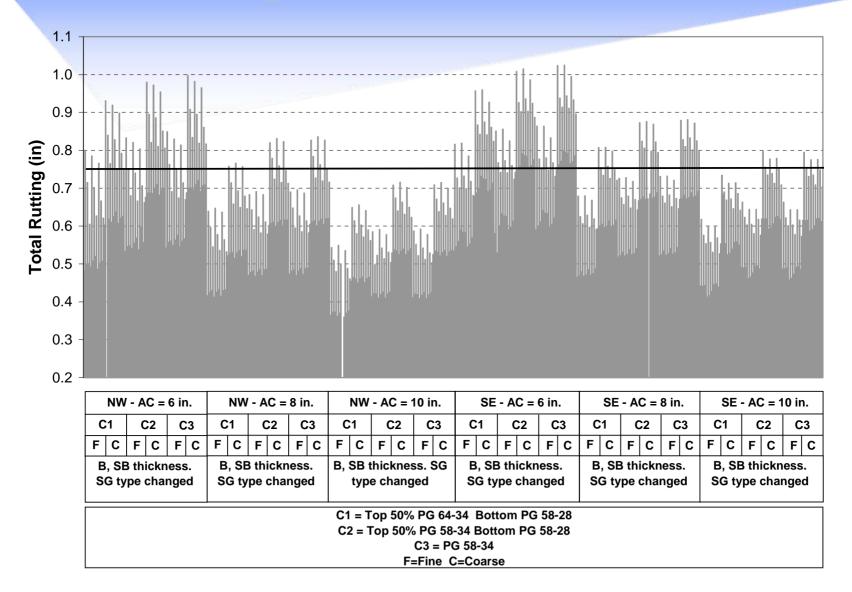
Sensitivity Analysis – Minnesota Condition

- Design factorial
 - 768 projects
 - Traffic: 2-levels
 - High, approximately 10-million ESALs (AADTT=2000)
 - Low, approximately 1-million ESALs (AADTT=200)
 - Climate: 2-levels
 - Northwest (Grand Forks, ND) and Southeast (Rochester, MN)
- Comparison of performance predictions

Alligator Cracking, V 0.900, 10 Million ESALs

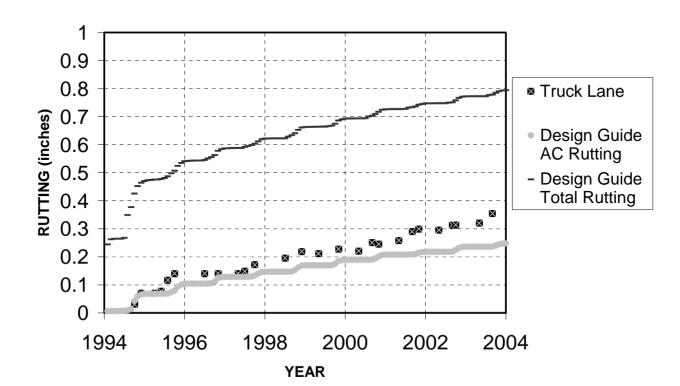


Total Rutting, V 0.900, 10 Million ESALs



Comparison with Measured Distresses

 MEPDG predictions were compared to the observed distresses for MnROAD cell 1 (5.9- in AC layer over a 33-in thick granular base resting on an A-6 subgrade)



Benefits of the 2002 Guide, cont.

- Accounts for many factors that change over time (traffic, climate, materials)
- Allows the prediction of key distress types as well as roughness over time
- Improved traffic characterization
- Improved structural modeling capabilities
- Improved materials characterization

Status of Design Guide

- 1st Version completed April 2004
- National review July 2004 December 2005
- Many software & engineering improvements by ARA & ASU.
- Latest Version 1.0 will be available
 April 1

More Information

Transportation Research Board web site: <u>www.trb.org/mepdg</u>

- Guide Documentation
- Software
- Climatic database